

Sapatas - Exemplo 1

SAPATAS

Neste exemplo, será dimensionada uma sapata utilizando como base a Norma ACI318-19, comparando o cálculo manual dela os resultados obtidos pelo TQS:

Dados:

Max axial service		
$N1 := 300.8 \cdot \text{kip}$	$Mx1 := 5.2 \cdot \text{kip} \cdot \text{ft}$	$Mz1 := 0 \cdot \text{kip} \cdot \text{ft}$
	$Vx1 := 0 \cdot \text{kip}$	$Vz1 := 0 \cdot \text{kip}$
Max moment service		
$N2 := 287.0 \cdot \text{kip}$	$Mx2 := 109.1 \cdot \text{kip} \cdot \text{ft}$	$Mz2 := 0 \cdot \text{kip} \cdot \text{ft}$
	$Vx2 := 0 \cdot \text{kip}$	$Vz2 := 0 \cdot \text{kip}$
Max factored axial		
$Nu1 := 394.5 \cdot \text{kip}$	$Mux1 := 7.2 \cdot \text{kip} \cdot \text{ft}$	$Muz1 := 0 \cdot \text{kip} \cdot \text{ft}$
	$Vux1 := 0 \cdot \text{kip}$	$Vuz1 := 0 \cdot \text{kip}$
Max factored moment		
$Nu2 := 312.3 \cdot \text{kip}$	$Mux2 := 264.6 \cdot \text{kip} \cdot \text{ft}$	$Muz2 := 0 \cdot \text{kip} \cdot \text{ft}$
	$Vux2 := 0 \cdot \text{kip}$	$Vuz2 := 0 \cdot \text{kip}$

Tensão Admissível no solo: 3,5 lb/ft² ou 24,306 psi

f_y : 60 ksi

f_c : 4 ksi

c: 3 in

Densidade do concreto: 150 lb/ft³

Pilar: 24 in x 24 in

Sapata:

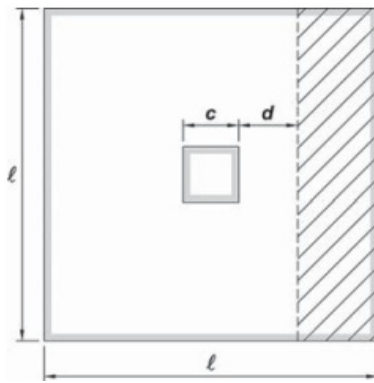
D: 10,5 ft

L: 10,5 ft

h_f : 25 in

q_u : 3,616 kip/ft²

Cortante:



$$V_n = V_c \rightarrow V_s = 0$$

$$d_b = \#6 = 0,75 \text{ in}$$

$$s = 10 \text{ in}$$

$$d = 25 - 3 - 0,75 = 21,25 \text{ in}$$

$$\rho_w = \frac{L}{s} \cdot \frac{d_b}{L \cdot d} = 0,003$$

$$\phi V_c = \phi \cdot 8 \cdot \lambda \cdot \rho_w^{\frac{1}{3}} \cdot \sqrt{f_c} \cdot b \cdot d = 0,75 \cdot 8 \cdot 1 \cdot \sqrt[3]{0,003} \cdot \sqrt{4000} \cdot 10,5 \cdot 12 \cdot 21,25 = 146,30 \text{ kips}$$

$$\phi V_{c,limite} = 5 \cdot 0,75 \cdot \sqrt{f'_c} \cdot b \cdot d = 5 \cdot 0,75 \cdot \frac{\sqrt{4000}}{1000} \cdot 10,5 \cdot 12 \cdot 21,25 = 635,02 \text{ kips} \rightarrow \phi V_{c,limite} \geq \phi V_c$$

$\rightarrow OK!!$

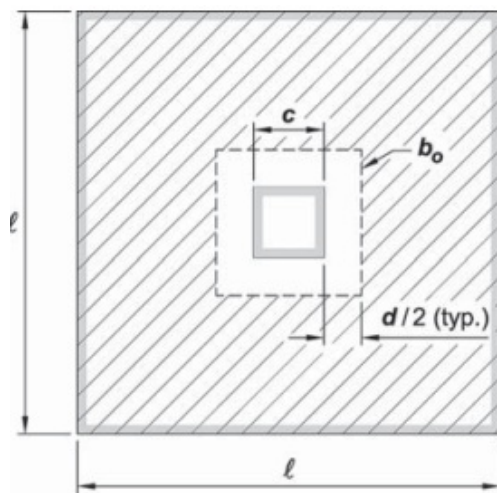
$$V_u = \left(\frac{L}{2} - \frac{a}{2} - d \right) \cdot b \cdot q_u = \left(\frac{10,5 \cdot 12}{2} - \frac{24}{2} - 21,25 \right) \cdot 10,5 \cdot 12 \cdot \frac{3,616}{144} = 94,13 \text{ kips}$$

$$V_u \leq \phi V_c \rightarrow OK!!$$

TQS:

Sentido	Seção (in)		Força (kip)		Aviso
	ds	bs	Atuante	Limite	
X+	20.648"	125.984"	94.74	143.12	OK
X-	20.648"	125.984"	94.74	143.12	OK
Y+	19.896"	125.984"	94.74	139.63	OK
Y-	19.896"	125.984"	94.74	139.63	OK

Punção:



$$b_o = 4 \cdot (c + d) = 4 \cdot (24 + 21,25) = 181 \text{ in}$$

$$v_{c1} = 4 \cdot \lambda \cdot \sqrt{f_c} = 253 \text{ psi} \quad v_{c1} = 4 \cdot \lambda \cdot \sqrt{f_c} = 253 \text{ psi}$$

$$v_{c2} = \left(2 + \frac{4}{\beta}\right) \cdot \lambda \cdot \sqrt{f_c} = 379,5 \text{ psi}$$

$$v_{c3} = \left(2 + \frac{\alpha \cdot d}{b_o}\right) \cdot \lambda \cdot \sqrt{f_c} = 423,50 \text{ psi}$$

$$v_{c1} \rightarrow OK!!$$

$$\phi V_c = \phi \cdot v_{c1} \cdot b_o \cdot d = 0,75 \cdot \frac{253}{1000} \cdot 181 \cdot 21,25 = 729,83 \text{ kips} \rightarrow 189,75 \text{ lb/in}^2$$

$$V_u = q_u \cdot [(a)^2 - (c + d)^2] = 3,616 \cdot \left[110,25 - \left(\frac{24 + 21,25}{12}\right)^2\right] = 347,25 \text{ kips} \rightarrow 90,28 \text{ lb/in}^2$$

$$V_u \leq \phi V_c \rightarrow OK!!$$

TQS:

Seção (in)		Tensão (lb/in ²)		Aviso
ds	b0	Atuante (vu)	Limite (ϕv_c)	
20.256"	178.583"	111.15	192.81	OK

Esmagamento:

$$B_n = \sqrt{\frac{A_2}{A_1}} \cdot 0,85 \cdot f_c \cdot A_1 = 5,25 \cdot 0,85 \cdot 4000 \cdot 576 = 10281 \text{ kips}$$

$$B_n = 2 \cdot (0,85 \cdot f'_c \cdot A_1) = 3916 \text{ kips}$$

$$\text{Na sapata: } \phi B_n = 0,65 \cdot 3916 = 2545,4 \text{ kips} \geq B_u (394,5 \text{ kips}) \rightarrow OK!!$$

$$\text{No pilar: } \phi B_n = 0,65 \cdot 1958 = 1272,7 \text{ kips} \geq B_u (394,5 \text{ kips}) \rightarrow OK!!$$

TQS:

Posição	Áreas (ft²)		Força (kip)		Aviso
	A1	A2	Atuante	Limite	
pilar	4.01	16.02	398.71	1359.71	OK
sapata	4.01	16.02	398.71	2719.43	OK

Flexão:

M_{ux} : 363,572 kip.ft - dx=21,625 in

M_{uy} : 339,317 kip.ft - dy=20,875 in

$$\phi M_n = M_u$$

$$a = \frac{A_s \cdot f_y}{0,85 \cdot f'_c \cdot b} = \frac{A_s \cdot 60}{0,85 \cdot 4 \cdot 10,5} = 0,17 A_s$$

$$A_{s,x} = \frac{M_u}{0,9 \cdot f_y \cdot (d - \frac{a}{2})} = \frac{363,572 \cdot 12}{0,9 \cdot 60 \cdot (21,625 - \frac{0,17 A_s}{2})} = 3,54 \text{ in}^2$$

$$A_{s,y} = \frac{M_u}{0,9 \cdot f_y \cdot (d - \frac{a}{2})} = \frac{339,317 \cdot 12}{0,9 \cdot 60 \cdot (20,875 - \frac{0,17 A_s}{2})} = 3,67 \text{ in}^2$$

$$A_{s,min} = \rho \cdot A_g = 0,0018 \cdot 25 \cdot 10,5 \cdot 12 = 5,67 \text{ in}^2$$

$$A_{s,min} \geq A_s \rightarrow A_{s,min}!!$$

Detalhamento:

Adotado #6 - 0,44 in²

$$\text{Número de Barras: } \frac{5,67}{0,442} = 12,82 \rightarrow 13 \text{ barras} \rightarrow s = \frac{10,5 \cdot 12}{13} = 9,69 \text{ in} \rightarrow \text{Adotado 9 in e 14 barras}$$

$$A_{s,efetivo,xy} = 14 \cdot 0,442 = 6,20 \text{ in}^2$$

TQS:

Sentido	Momentos (kip-ft)		Armaduras (in²)					
	M _{sd}	M _{min}	d	A _{s,calc}	A _{s,calc,corr}	A _{s,min,rho}	A _{s,min,crit}	A _{s,nec}
X	342.61	0.00	20.632"	4.17	4.17	5.45	0.23	5.45
Y	342.61	0.00	19.880"	4.32	4.32	5.45	0.23	5.45

Sentido	Armaduras				
	AS _{det} (in ²)	AS _{det/s} (in ² /ft)	N _Ø	Ø (in)	c/ (in)
X	6.22	0.59	14	0.752"	9.055"
Y	6.22	0.59	14	0.752"	9.055"

Comprimento de Ancoragem:

$$\ell_d = \left(\frac{3}{40} \cdot \frac{f_y}{\lambda \cdot \sqrt{f'_c}} \cdot \frac{\psi_t \cdot \psi_e \cdot \psi_s \cdot \psi_g}{\left(\frac{c_b + K_{tr}}{d_b} \right)} \right) \cdot d_b = \left(\frac{3}{40} \cdot \frac{60000}{1 \cdot \sqrt{4000}} \cdot \frac{1 \cdot 1 \cdot 0,8 \cdot 1}{\left(\frac{3,5 + 0}{0,75} \right)} \right) \cdot 0,75 = 12,2 \text{ in}$$

$$\ell_{d,prov} = \frac{10,5 \cdot 12}{2} - 24 - cover = 36 \text{ in} \geq 12 \text{ in} \rightarrow OK!!$$

Arranques:

Comprimento dos arranques do pilar:

$$A_{s,dowel} = 0,005 \cdot A_g = 0,005 \cdot 24^2 = 2,88 \text{ in}^2$$

$$\ell_{dc1} = \frac{f_y \cdot \psi_r}{50 \cdot \lambda \cdot \sqrt{f'_c}} \cdot d_b = \frac{60000 \cdot 1}{50 \cdot 1 \cdot \sqrt{4000}} \cdot 0,75 = 14,23 \text{ in}$$

$$\ell_{dc2} = 0,0003 \cdot f_y \cdot \psi_r \cdot d_b = 0,0003 \cdot 60000 \cdot 1 \cdot 0,75 = 13,5 \text{ in}$$

$$\text{Adotado } \ell_{dc1} \rightarrow 14,23 \text{ in}$$

$$\ell_{sc1} = 0,0005 \cdot f_y \cdot d_b = 0,0005 \cdot 60000 \cdot 0,75 = 22,5 \text{ in}$$

$$\ell_{sc2} = 12 \text{ in}$$

$$\text{Adotado } \ell_{sc1} \rightarrow 22,5 \text{ in}$$

Comprimento total:

$$\ell_{dc1} + \ell_{sc1} = 14,23 + 22,5 = 36,73 \text{ in}$$

TQS:

Technical drawing of a reinforced concrete slab and its cross-sections.

Top View:

- Overall dimensions: $10'-5 \frac{31}{32}"$ by $10'-5 \frac{31}{32}"$.
- Central square opening: $2'-1 \frac{1}{32}"$ by $2'-1 \frac{1}{32}"$.
- Reinforcement: $14 \text{ N2 } @ 9.055"$ (top) and $14 \text{ N1 } @ 9.055"$ (bottom).

Cross-Section (Left):

- Overall height: $16.93"$.
- Clear height: $120.00"$.
- Top width: $2'-1 \frac{1}{32}"$.
- Bottom width: $2'-1 \frac{1}{32}"$.
- Radius: $R=2.256"$.
- Label: **PREVER ARRANQUES** (cut bars).

Cross-Section (Right):

- Overall height: $16.93"$.
- Clear height: $120.00"$.
- Top width: $2'-1 \frac{1}{32}"$.
- Bottom width: $2'-1 \frac{1}{32}"$.
- Radius: $R=2.256"$.
- Label: **PREVER ARRANQUES** (cut bars).

Reinforcement Details:

- $\text{N1 } 14 \text{ } \varnothing \text{ #6 } @ 9.055" \text{ } l=155.44"$
- $\text{N2 } 14 \text{ } \varnothing \text{ #6 } @ 9.055" \text{ } l=153.86"$



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