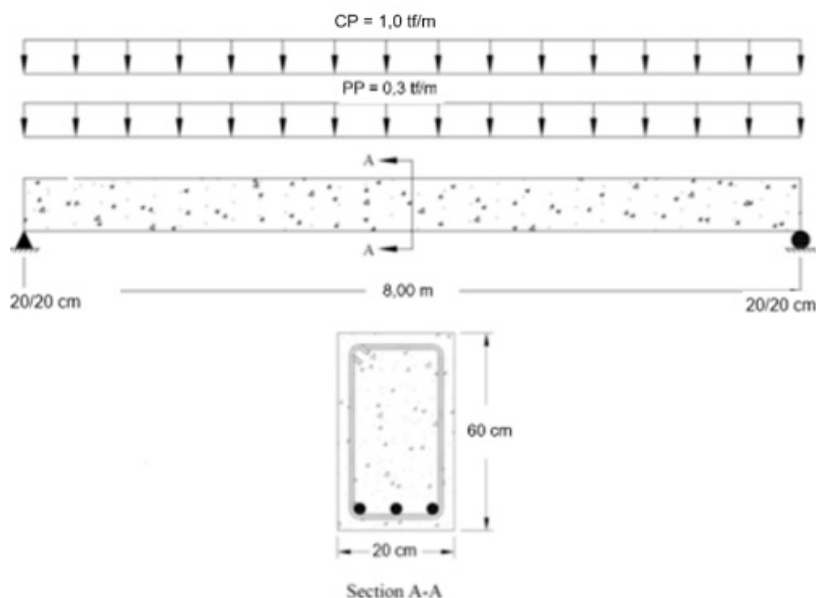


Vigas - Flexão Simples 3

FLEXÃO SIMPLES

Neste exemplo, será dimensionada a armadura longitudinal e transversal de uma viga submetida a Flexão Simples utilizando como base a Norma ACI318-19, conforme dados abaixo:

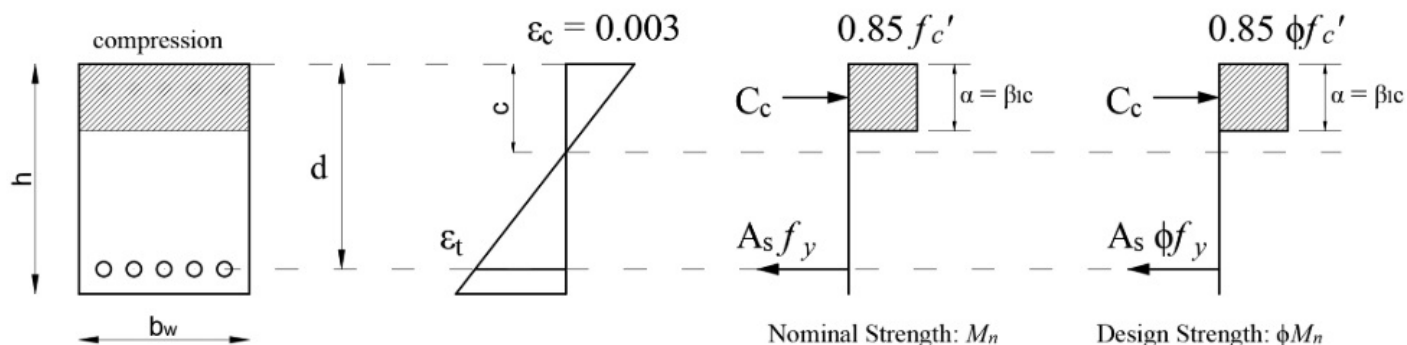


Concreto - C25 (4000 psi) / Aço – ADN420 (60 ksi)

b_w : 8 in / h : 24 in

M_u :

TQS = 1124,08 kip.in	Software B = 1177,18 kip.in	Software C = 1141,78 kip.in
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Flexão:

$$M_n = \frac{M_u}{\phi} = \frac{1124,08}{0,9} = 1248,83 \text{ kip.in}$$

$$d = h - C_c - d_{be} - \frac{d_b}{2} = 24 - 1,57 - 0,375 - 0,25 = 21,81 \text{ in}$$

$$R_n = \frac{M_{n,req}}{b_w \cdot d^2} = \frac{1248,83}{8 \cdot 21,81^2} = 0,328 \text{ ksi} = 328,20 \text{ psi}$$

$$\rho = \left(0,85 \cdot \frac{f'_c}{f_y}\right) \cdot \left[1 - \sqrt{1 - \left(\frac{2 \cdot R_n}{0,85 \cdot f'_c}\right)}\right] = \left(0,85 \cdot \frac{4}{60}\right) \cdot \left(1 - \sqrt{1 - \left(\frac{2 \cdot 0,328}{0,85 \cdot 4}\right)}\right) = 0,0058$$

$$A_s = \rho \cdot b_w \cdot d = 0,0058 \cdot 8 \cdot 21,81 = 1,01 \text{ in}^2$$

As,min (ACI 318-19 – 9.6.1.2)

$$\left(\frac{3 \cdot \sqrt{f'_c}}{f_y}\right) \cdot b_w \cdot d = \left(\frac{3 \cdot \sqrt{4000}}{60000}\right) \cdot 8 \cdot 21,81 = 0,55 \text{ in}^2$$

$$\left(\frac{200}{f_y}\right) \cdot b_w \cdot d = \left(\frac{200}{60000}\right) \cdot 8 \cdot 21,81 = 0,58 \text{ in}^2$$

$$A_{s,min} = 0,58 \text{ in}^2$$

TQS = 1,01 in2	Software B = 1,13 in2	Software C = 0,99 in2
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1,00 in2 ≈ 1,01 in2 → OK!

5#4 → 1,00 in2

øMn (1118,18 kip.in) ≈ Mu (1124,08 kip.in) → OK!!

Verificação do fator de redução (ACI318-19 – 21.2.2)

$$\alpha = \frac{A_s \cdot f_y}{0,85 \cdot f'_c \cdot b_w} = \frac{1,01 \cdot 60}{0,85 \cdot 4 \cdot 8} = 2,22 \text{ in}$$

$$\text{Linha Neutra: } c = \frac{\alpha}{\beta_1} = \frac{2,14}{0,85} = 2,61 \text{ in}$$

$$\varepsilon_t = 0,003 \cdot \left(\frac{d - c}{c}\right) = 0,003 \cdot \left(\frac{21,81 - 2,61}{2,61}\right) = 0,0221$$

0,0221 ≥ 0,005 → OK! Tension Controlled! → ø 0,90 OK!

Cortante:

TQS	Software B	Software C
Vu = 16,07 kips	Vu = 15,66 kips	Vu = 15,99 kips
Vu,design = 13,40 kips	Vu,design = 13,36 kips	Vu,design = 13,45 kips

$$V_n = V_c + V_s$$

$$V_u = 16,07 \text{ kips}$$

$$V_{u,design} = 13,40 \text{ kips}$$

$$V_c = 2 \cdot \lambda \cdot \sqrt{f'_c} \cdot b_w \cdot d = 2 \cdot 1 \cdot \sqrt{4000} \cdot 8 \cdot 21,81 = 22,07 \text{ kips}$$

$$V_n = \frac{V_{u,design}}{\phi_{0,75}} = V_c + V_s \rightarrow V_s = 0 \text{ kips} \rightarrow V_n = 17,87 \text{ kips}$$

$$V_{s,lim} = 8 \cdot \sqrt{f'_c} \cdot b_w \cdot d = 8 \cdot \sqrt{4000} \cdot 8 \cdot 21,81 = 88,28 \text{ kips}$$

$V_s \leq V_{s,lim}$ OK!

$\phi_{0,75} \rightarrow \phi V_n \geq V_u \rightarrow 13,40 \geq 13,40 \rightarrow \text{OK!!}$

Área de Aço:

$$A_v = \frac{V_s \cdot s}{f_{yt} \cdot d} = \frac{A_v}{s} = \frac{0}{60 \cdot 21,81} = 0,0 \text{ in}^2$$

$$\frac{A_{v,min}}{s} = \frac{0,75 \cdot \sqrt{f'_c} \cdot b_w}{f_{yt}} = 0,0063 \text{ in}^2$$

$$\frac{A_{v,min}}{s} = \frac{50 \cdot b_w}{f_{yt}} = 0,0067 \text{ in}^2$$

$$A_v \geq A_{v,min} \rightarrow 0,00 \leq 0,0067 \rightarrow A_{v,min}!!$$

Espaçamento:

$$s \leq s_{max} \rightarrow s_{max} = \text{menor valor entre } s_1 \text{ e } s_2$$

$$s_1 = \frac{d}{2} = 10,90 \text{ in ou } s_2 = 24 \text{ in} \rightarrow 10,90 \text{ in!} \rightarrow 10 \text{ in!!}$$

$$\phi_t = \#3 \rightarrow \#3 @ 10 \text{ in}$$

$\phi V_n (23,89 \text{ kips}) \geq V_u (13,40 \text{ kips})$

